

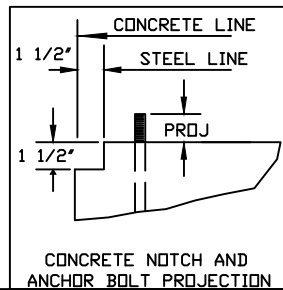
**GENERAL NOTES**

FOUNDATION DESIGN AND CONSTRUCTION ARE NOT THE RESPONSIBILITY OF S&S STEEL BUILDINGS.

THE BUILDING REACTION DATA REPORTS THE LOADS WHICH THIS BUILDING PLACES ON THE FOUNDATION.

ANCHOR BOLTS SHALL BE ACCURATELY SET TO A TOLERANCE OF +/- 1/8" IN BOTH ELEVATION AND LOCATION. ANCHOR BOLTS ARE TO BE TYPE A36.

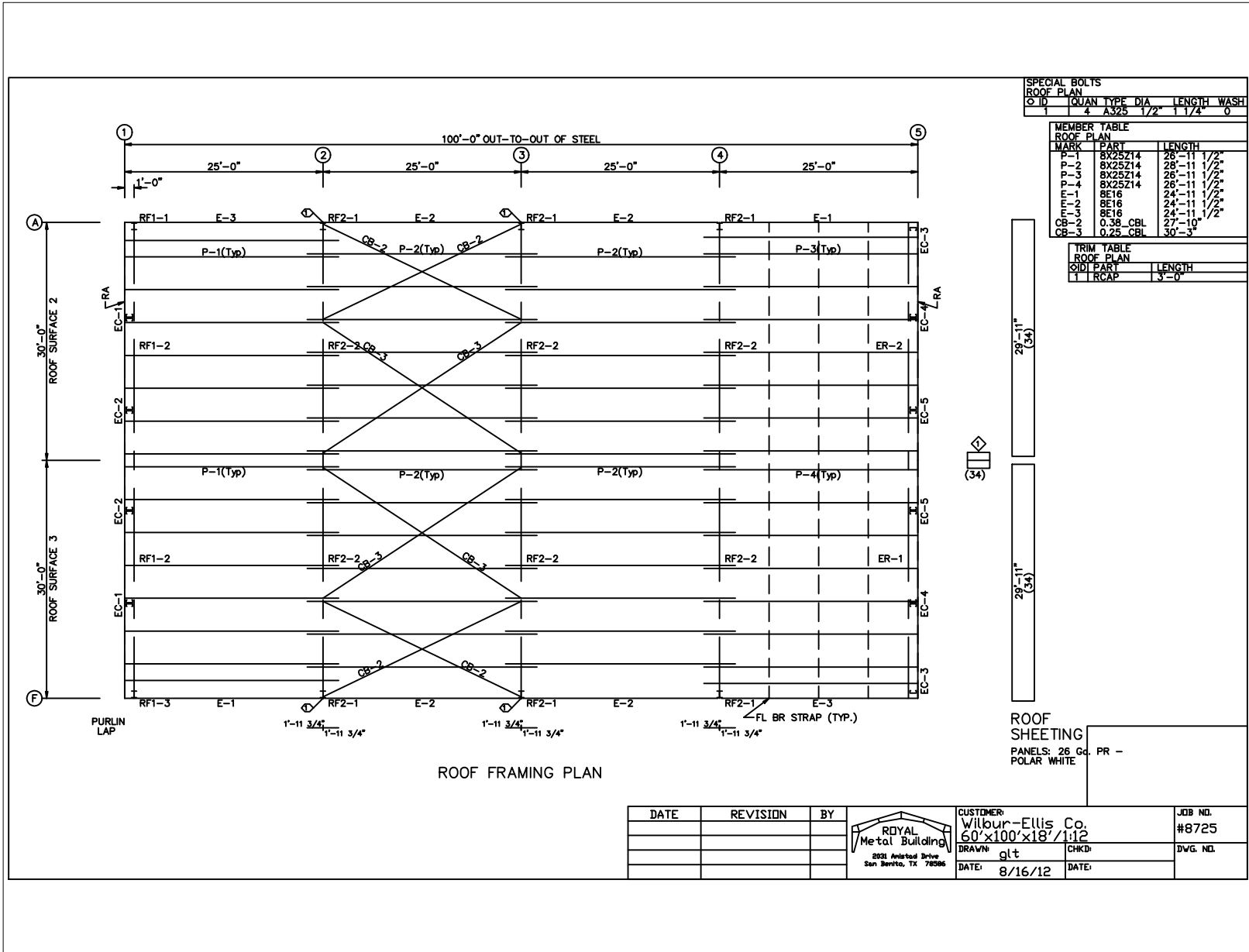
COLUMN BASE PLATES ARE DESIGNED NOT TO EXCEED A BEARING PRESSURE OF 1125 POUNDS PER SQUARE INCH.



**ANCHOR BOLT SUMMARY**

Qty	Locate	Dia (in)	Type	Proj (in)
0	Jamb			
40	Endwall	5/8"	A307	3.00
32	Frame	3/4"	A307	3.00

DATE	REVISION	BY	CUSTOMER: Wilbur-Ellis Co. 60'x100'x18'/1:12		JOB NO. #8725
			ROYAL Metal Building 2821 Aristed Drive San Benito, TX 78586	DRAWN: glt	CHKD:
				DATE: 8/16/12	DATE:
					DWG. NO.



SPECIAL BOLTS  
ROOF PLAN

Q ID	QUAN	TYPE	DIA	LENGTH	WASH
1	4	A325	1/2"	1 1/4"	0

MEMBER TABLE  
ROOF PLAN

MARK	PART	LENGTH
P-1	8X25Z14	28'-11 1/2"
P-2	8X25Z14	28'-11 1/2"
P-3	8X25Z14	28'-11 1/2"
P-4	8X25Z14	28'-11 1/2"
E-1	BE16	24'-11 1/2"
E-2	BE16	24'-11 1/2"
E-3	BE16	24'-11 1/2"
CB-2	0.38 CBL	27'-10"
CB-3	0.25 CBL	30'-3"

TRIM TABLE  
ROOF PLAN

QIDT PART	LENGTH
1 RCAP	3'-0"

29'-11" (34)

29'-11" (34)

ROOF FRAMING PLAN

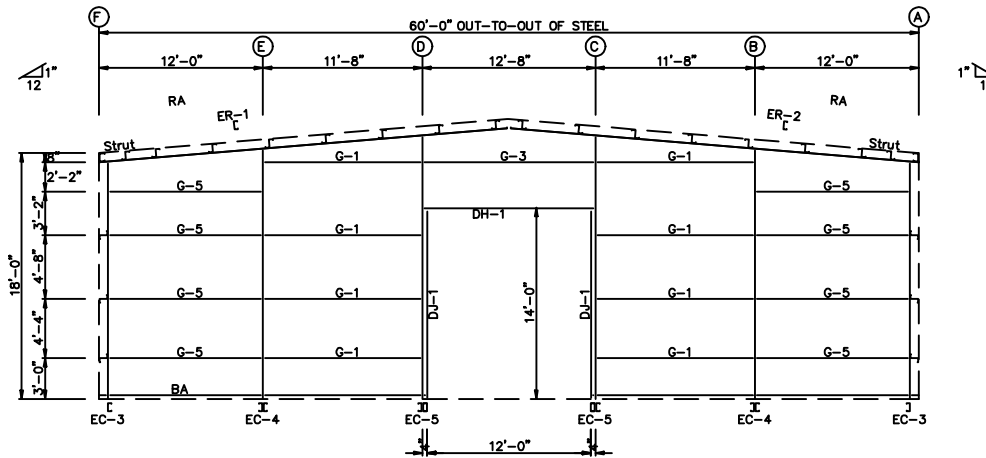
ROOF SHEETING  
PANELS: 26 Gd. PR -  
POLAR WHITE

DATE	REVISION	BY



CUSTOMER: Wilbour-Ellis Co.  
60'x100'x18'/1:12  
DRAWN: glt  
DATE: 8/16/12

JOB NO. #8725  
DWG. NO.



**BOLT TABLE**

FRAME LINE 5				
LOCATION	QUAN	TYPE	DIA	LENGTH
ER-1/ER-2	4	A325	5/8"	1 3/4"
Cor_Column/Raf	2	A325	1/2"	1 1/4"
Int_Column/Raf	4	A325	1/2"	1 1/4"

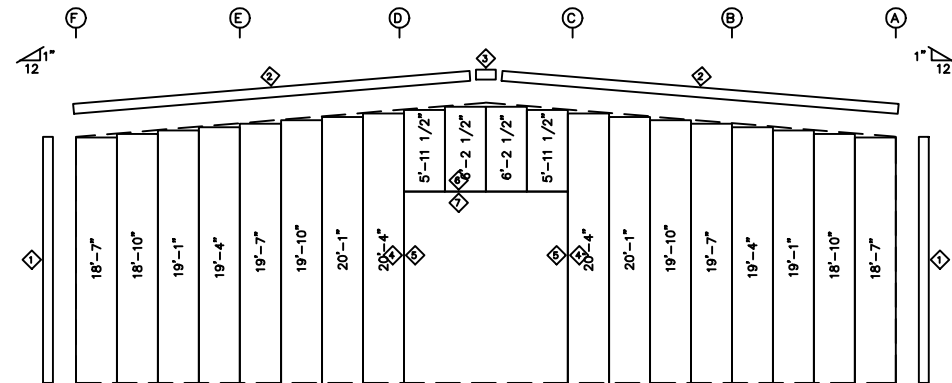
**MEMBER TABLE**

FRAME LINE 5		
MARK	PART	LENGTH
EC-3	8X35C16	17'-3 9/16"
EC-4	8X70D14	18'-2 9/16"
EC-5	8X70D12	19'-2 3/16"
ER-1	10X35C14	29'-9 7/8"
ER-2	10X35C14	29'-9 7/8"
DJ-1	8X35C16	13'-11 3/4"
DH-1	8X35C16	11'-11 1/2"
G-1	8X25Z16	10'-11 1/2"
G-3	8X25Z16	11'-11 1/2"
G-5	8X25Z16	10'-7 1/2"

**TRIM TABLE**

FRAME LINE 5	
QID/PART	LENGTH
1 CT-1	18'-1"
2 RT-1	19'-5"
3 PB-1	1'-8"
4 JC-1	14'-2"
5 JT-1	14'-2"
6 HC-1	12'-3"
7 DT-1	12'-3"

ENDWALL FRAMING: FRAME LINE 5



ENDWALL SHEETING & TRIM: FRAME LINE 5

PANELS: 26 Ga. R - AZTEC GOLD

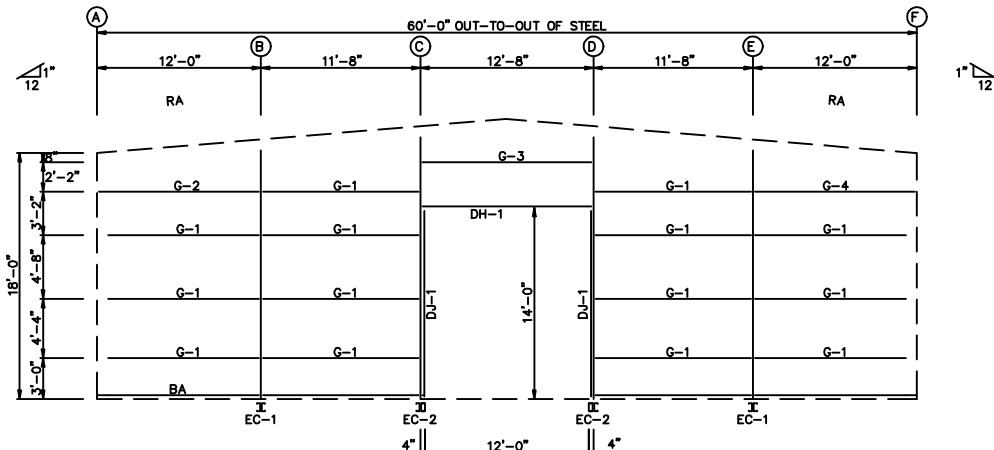
WALL PANELS TO FLOOR 6" LONGER THAN NORMAL.

DATE	REVISION	BY

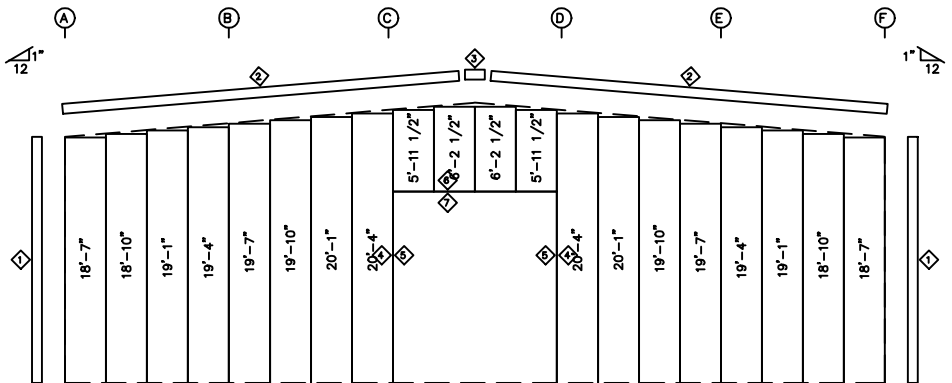


CUSTOMER:		Wiltour-Ellis Co.	
DRAWN:		60'x100'x18'/1:12	
DATE:	8/16/12	CHKD:	
		DATE:	

JOB NO.	#8725
DWG. NO.	



ENDWALL FRAMING: FRAME LINE 1



ENDWALL SHEETING & TRIM: FRAME LINE 1

PANELS: 26 Ga. R - AZTEC GOLD

WALL PANELS TO FLOOR 6" LONGER THAN NORMAL.

BOLT TABLE			
FRAME LINE 1			
LOCATION	QUAN	TYPE	DIA
Columns/Raf	B	A325	1/2" 1 1/4"

MEMBER TABLE		
FRAME LINE 1		
MARK	PART	LENGTH
EC-1	8X70D14	18'-1 15/16"
EC-2	8X70D14	19'-1 5/8"
DJ-1	8X35C16	13'-11 3/4"
DH-1	8X35C16	11'-11 1/2"
G-1	8X25Z16	10'-11 1/2"
G-2	8X25Z16	11'-7 1/2"
G-3	8X25Z16	11'-11 1/2"
G-4	8X25Z16	11'-7 1/2"

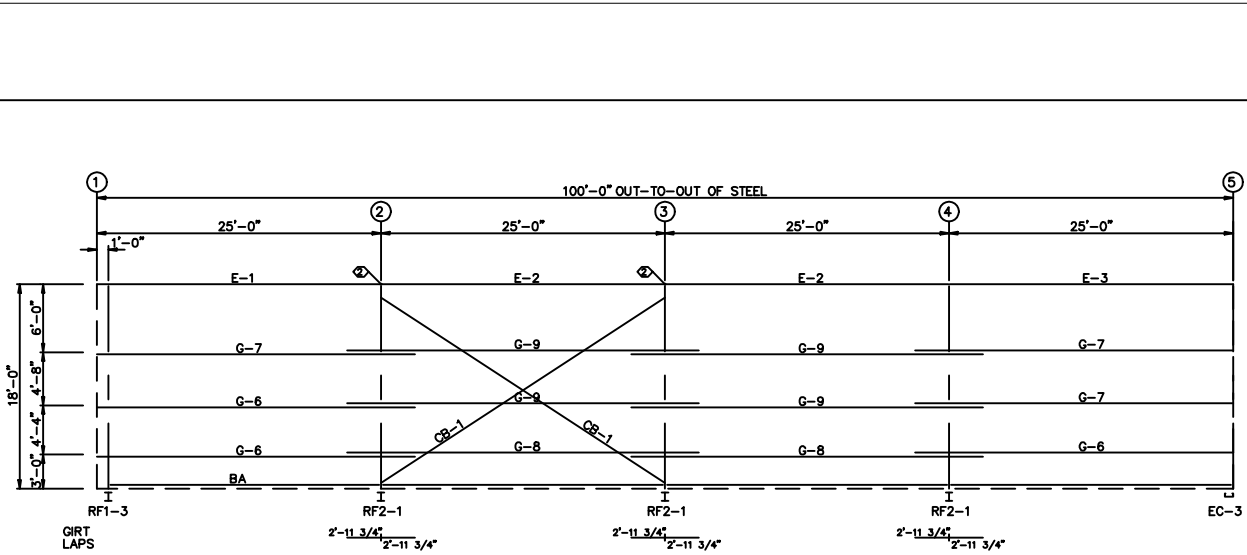
TRIM TABLE	
FRAME LINE 1	
QID	PART
1	CT-1
2	RT-1
3	PB-1
4	JC-1
5	JT-1
6	HC-1
7	DT-1

DATE	REVISION	BY

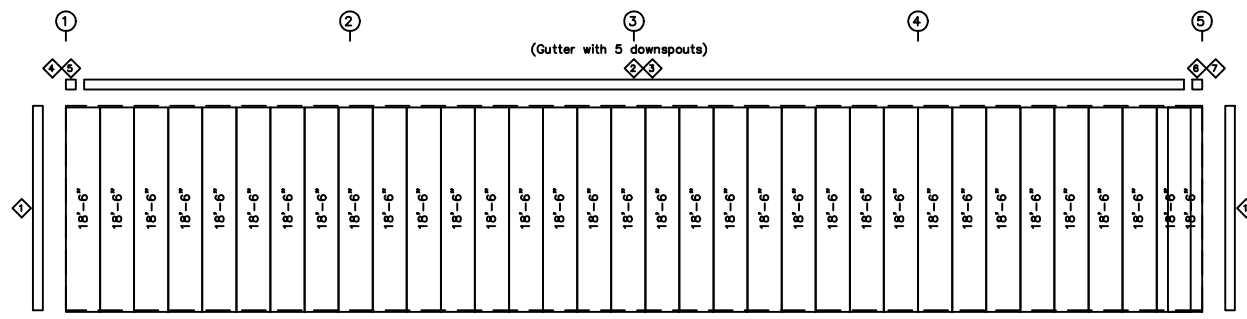


CUSTOMER: Wilbour-Ellis Co.  
60'x100'x18'/1:12  
DRAWN: glt  
DATE: 8/16/12

JOB NO. #8725  
DWG. NO.



SIDEWALL FRAMING: FRAME LINE F



SIDEWALL SHEETING & TRIM: FRAME LINE F  
PANELS: 26 Ga. R - AZTEC GOLD

WALL PANELS TO FLOOR 6" LONGER THAN NORMAL.

SPECIAL BOLTS				
C ID	QUAN	TYPE	DIA	LENGTH WASH
2	4	A325	1/2"	1 1/4"

MEMBER TABLE		
FRAME LINE F		
MARK	PART	LENGTH
E-1	BE16	24'-11 1/2"
E-2	BE16	24'-11 1/2"
E-3	BE16	24'-11 1/2"
G-6	8X25Z14	27'-11 1/2"
G-7	8X25Z12	27'-11 1/2"
G-8	8X25Z16	30'-11 1/2"
G-9	8X25Z14	30'-11 1/2"
CB-1	0.50 CBL	30'-5"

TRIM TABLE		
FRAME LINE F		
SUBJ	PART	LENGTH
1	CT-1	18'-1"
2	GU-1	20'-4"
4	CS-1	9"
5	SCB-L	1/8"
6	SCB-R	1/8"
7	GEC-R	1/8"

DATE	REVISION	BY



CUSTOMER: Wilbour-Ellis Co.  
60'x100'x18'/1:12  
DRAWN: glt  
DATE: 8/16/12

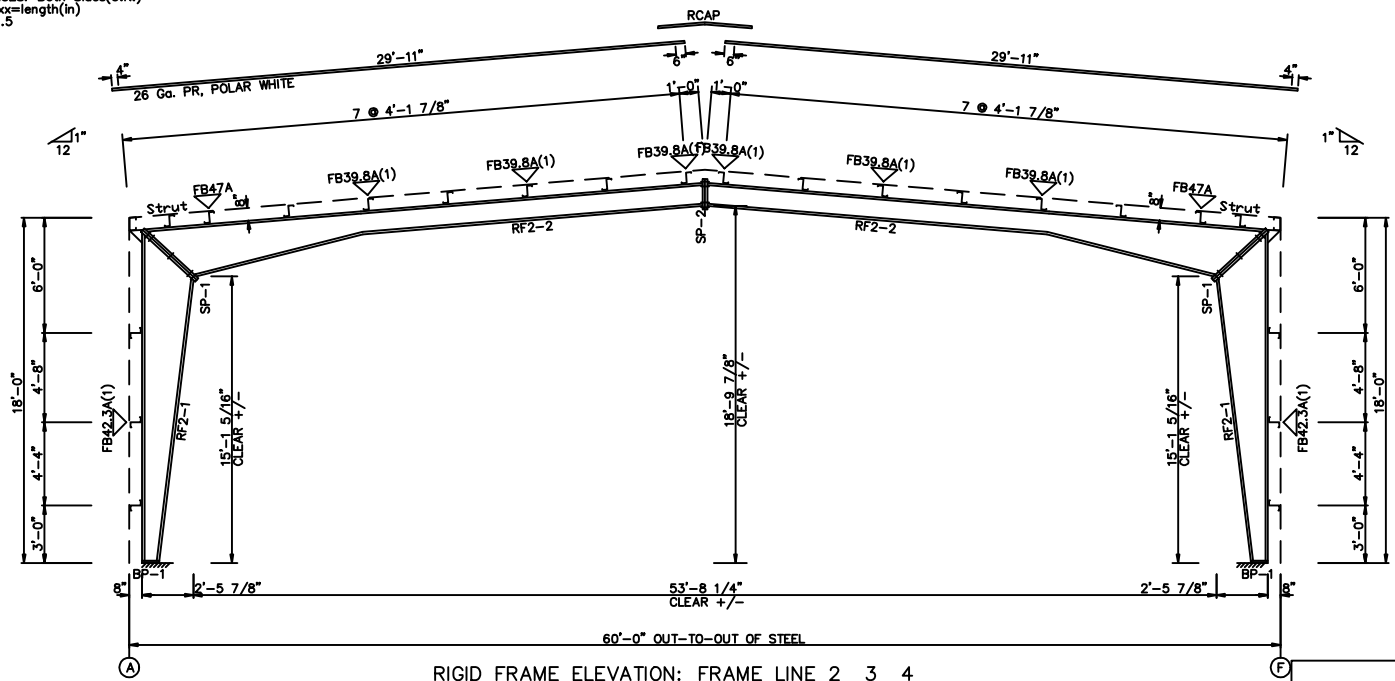
JOB NO. #8725  
DWG. NO.

SPLICE BOLT TABLE						
Mark	Qty		Int	Type	Dia	Length
	Top	Bot				
SP-1	4	4	2	A325	0.750	2.75
SP-2	4	4	0	A325	0.750	2.00

BASE PLATE TABLE			
Col	Mark	Plate Size	
		Width	Length
	BP-1	8"	8 1/8"

MEMBER TABLE						
Mark	Web		Web Plate		Outside Flange	Inside Flange
	Depth	Thick	Thick	Length		
RF2-1	7.5/16.2	0.135	0.135	71.0	5 x 1/4" x 206.9	5 x 1/4" x 181.0
	16.2/28.4	0.188	0.188	135.9		
RF2-2	29.4/11.5	0.188	0.135	136.0	5 x 1/4" x 351.4	5 x 1/4" x 110.2
	11.5/11.5	0.135	0.135	142.0		
				73.5		

FLANGE BRACES: Both Sides(U.N.)  
 FBxxA(1); xx=length(in)  
 A - L1,Sx1.5



RIGID FRAME ELEVATION: FRAME LINE 2 3 4

DATE	REVISION	BY	CUSTOMER:		JOB NO.
			Willour-Ellis Co.		#8725
			60'x100'x18' 1:12		DWG. NO.
			DRAWN: glt	CHKD:	
			DATE: 8/16/12	DATE:	

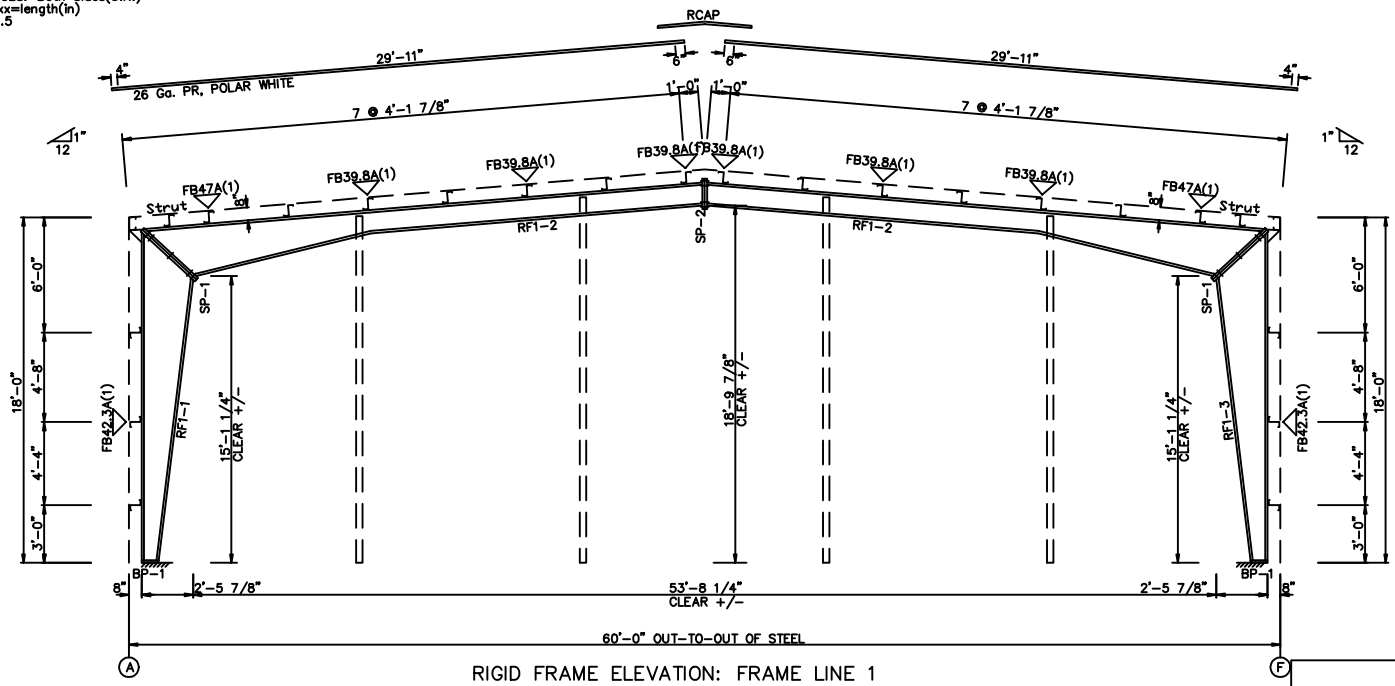


SPLICE BOLT TABLE						
Mark	Qty		Int	Type	Dia	Length
	Top	Bot				
SP-1	4	4	2	A325	0.750	2.75
SP-2	4	4	0	A325	0.750	2.00

BASE PLATE TABLE			
Col Mark	Plate Size		Length
	Width	Thick	
BP-1	8"	1/2"	8 1/8"

MEMBER TABLE					Outside Flange W x Thk x Length	Inside Flange W x Thk x Length
Mark	Web Depth	Web Plate	Start/End	Thick		
RF1-1	7.6/16.2	0.135	71.0		5 x 1/4" x 206.9	5 x 1/4" x 181.0
RF1-2	16.2/29.4	0.188	135.9		5 x 1/4" x 351.4	5 x 1/4" x 115.4
	29.4/11.5	0.188	141.2			
	11.5/11.5	0.135	142.0			
RF1-3	11.5/11.5	0.135	68.2		5 x 1/4" x 206.9	5 x 1/4" x 181.0
	29.4/16.2	0.188	135.9			
	16.2/7.6	0.135	71.0			

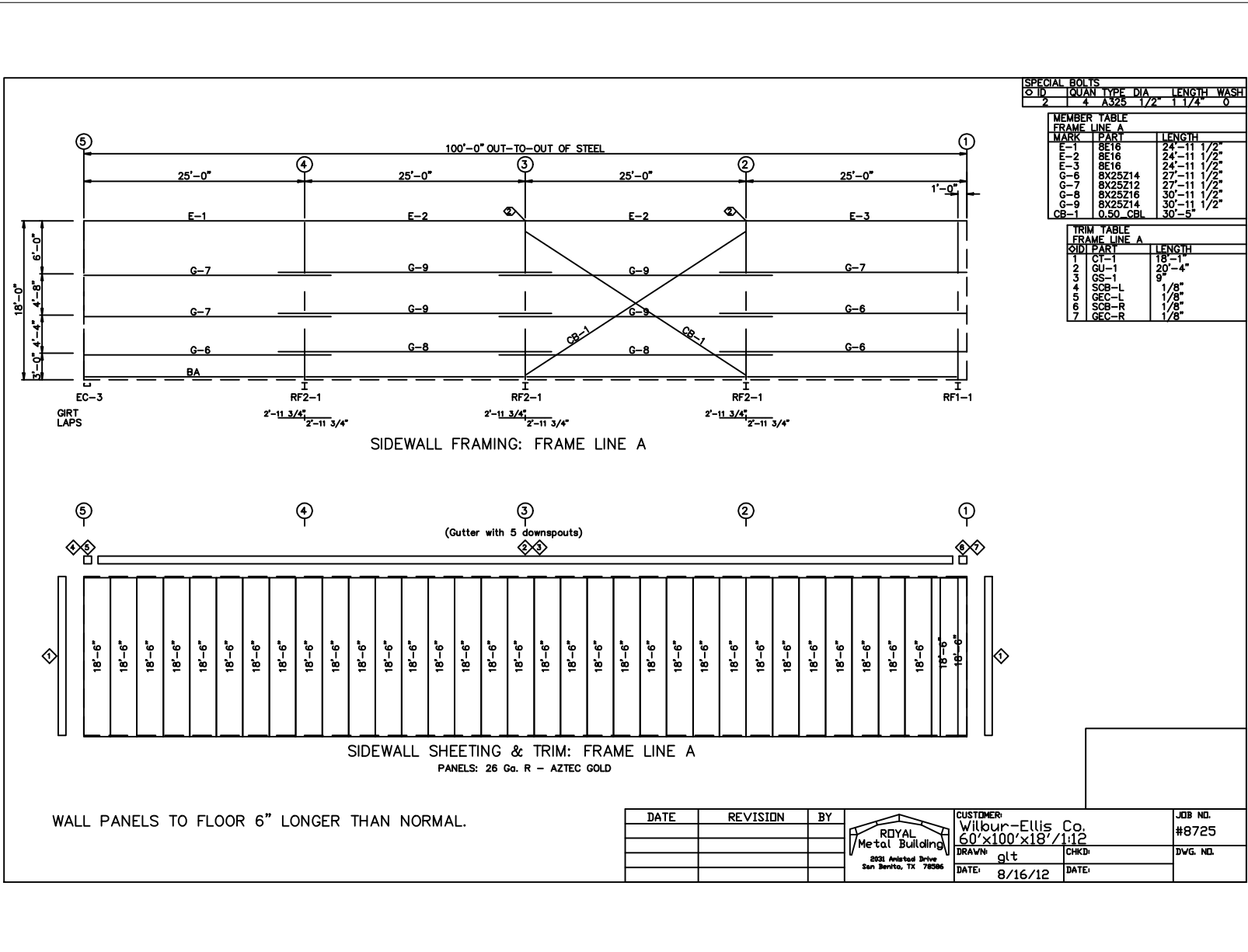
FLANGE BRACES: Both Sides(U.N.)  
 FBxxA(1): xx=length(in)  
 A - L1.5x1.5



DATE	REVISION	BY	CUSTOMER:		JOB NO.
			Wiltour-Ellis Co.		#8725
			60'x100'x18'/1:12		DWG. NO.
			DRAWN: glt	CHKD:	
			DATE: 8/16/12	DATE:	







SPECIAL BOLTS				
C ID	QUAN	TYPE	DIA	LENGTH WASH
2	4	A325	1/2"	1 1/4" 0

MEMBER TABLE		
FRAME LINE A		
MARK	PART	LENGTH
E-1	BE16	24'-11 1/2"
E-2	BE16	24'-11 1/2"
E-3	BE16	24'-11 1/2"
G-6	8X25Z14	27'-11 1/2"
G-7	8X25Z12	27'-11 1/2"
G-8	8X25Z16	30'-11 1/2"
G-9	8X25Z14	30'-11 1/2"
CB-1	0.50 CBI	30'-5"


TRIM TABLE		
FRAME LINE A		
SUBJ	PART	LENGTH
1	CT-1	18'-1"
2	GU-1	20'-4"
4	SCB-L	9"
5	GEC-L	1/8"
6	SCB-R	1/8"
7	GEC-R	1/8"

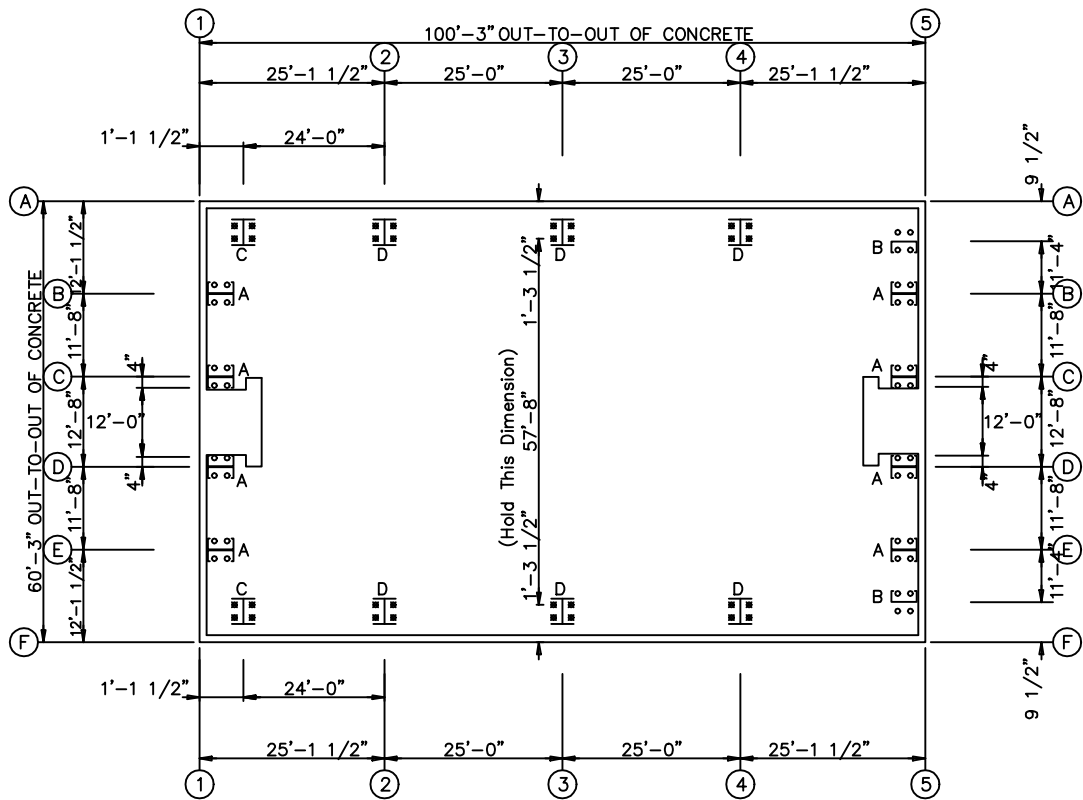
SIDEWALL FRAMING: FRAME LINE A

SIDEWALL SHEETING & TRIM: FRAME LINE A

PANELS: 26 Ga. R - AZTEC GOLD

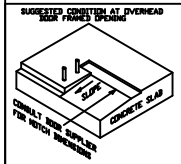
WALL PANELS TO FLOOR 6" LONGER THAN NORMAL.

DATE	REVISION	BY	 2021 Ansted Drive San Benito, TX 78586	CUSTOMER:	JOB NO.
				Wilbur-Ellis Co. 60'x100'x18'/1:12	#8725
			DRAWN: glt DATE: 8/16/12	CHKD: DATE:	DWG. NO.



- Dia= 5/8"
- Dia= 3/4"

ANCHOR BOLT PLAN  
 NOTE: All Base Plates @ 100'-0" (U.N.)



DATE	REVISION	BY



CUSTOMER: Wilbour-Ellis Co.  
 60'x100'x18'/1:12  
 DRAWN: glt  
 DATE: 8/16/12

JOB NO. #8725  
 DWG. NO.  
 DATE:

ENDWALL COLUMN: BASIC COLUMN REACTIONS (k )

Frm Line	Col Line	Dead Vert	Wind_P Horz	Wind_S Horz
1	B	0.1	-2.3	2.5
1	C	0.1	-2.5	2.7
1	D	0.1	-2.5	2.7
1	E	0.1	-2.3	2.5

Frm Line	Col Line	Dead Vert	Collat Vert	Live Vert	Rafter Wind_L Vert	Rafter Wind_R Vert	Brace Wind_L Vert	Brace Wind_R Vert	Wind_P Horz	Wind_S Horz	LnWind1 Vert	LnWind2 Vert	Seis_L Vert
5	F	0.2	0.2	1.3	-1.9	-1.2	-1.9	-1.2	0.0	0.0	-1.5	-0.9	0.0
5	E	0.5	0.4	3.0	-4.6	-2.5	-4.6	-2.5	-2.3	2.5	-3.4	-2.0	0.0
5	D	0.6	0.5	3.3	-4.6	-3.0	-4.6	-3.0	-2.6	2.8	-3.6	-2.1	0.0
5	C	0.6	0.5	3.3	-3.0	-4.6	-3.0	-4.6	-2.6	2.8	-3.6	-2.1	0.0
5	B	0.5	0.4	3.0	-2.5	-4.6	-2.5	-4.6	-2.3	2.5	-3.4	-2.0	0.0
5	A	0.2	0.2	1.3	-1.2	-1.9	-1.2	-1.9	0.0	0.0	-1.5	-0.9	0.0

Frm Line	Col Line	Seis_R Vert	-LWIND1_L- Horz	-LWIND1_L- Vert	-LWIND1_R- Horz	-LWIND1_R- Vert	-LWIND2_L- Horz	-LWIND2_L- Vert	-LWIND2_R- Horz	-LWIND2_R- Vert
5	F	0.0	0.0	-0.7	0.0	0.0	0.0	-0.7	0.0	0.0
5	E	0.0	0.0	-0.9	0.0	0.0	0.0	-0.9	0.0	0.0
5	D	0.0	0.0	0.1	0.0	0.0	0.0	0.1	0.0	0.0
5	C	0.0	0.0	0.0	0.0	0.1	0.0	0.0	0.0	0.1
5	B	0.0	0.0	0.0	0.0	-0.9	0.0	0.0	0.0	-0.9
5	A	0.0	0.0	0.0	0.0	-0.7	0.0	0.0	0.0	-0.7

ENDWALL COLUMN: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES


Frm Line	Col Line	Load ID	Column Reactions (k )			Anc. Qty	Bolt Dia	Base_Plate (in)			Grout (in)		
			Load H	V Hmax	V Hmin			Width	Length	Thick			
1	B	8	2.5	0.1	9	-2.3	0.1	4	0.625	7.000	6.000	0.188	0.0
		10	1.9	0.1	9	-2.5	0.1	4	0.625	7.000	6.000	0.188	0.0
1	C	8	2.7	0.1	9	-2.5	0.1	4	0.625	7.000	6.000	0.188	0.0
		10	2.0	0.1	9	-2.5	0.1	4	0.625	7.000	6.000	0.188	0.0
1	D	8	2.7	0.1	9	-2.5	0.1	4	0.625	7.000	6.000	0.188	0.0
		10	2.0	0.1	9	-2.5	0.1	4	0.625	7.000	6.000	0.188	0.0
1	E	8	2.5	0.1	9	-2.3	0.1	4	0.625	7.000	6.000	0.188	0.0
		10	1.9	0.1	9	-2.3	0.1	4	0.625	7.000	6.000	0.188	0.0
5	F	11	0.0	-1.8	11	0.0	-1.8	4	0.625	7.000	6.000	0.188	0.0
		1	0.0	1.7	11	0.0	1.7	4	0.625	7.000	6.000	0.188	0.0
5	E	11	2.5	-4.3	12	-2.3	-3.1	4	0.625	7.000	6.000	0.188	0.0
		1	0.0	3.9	11	2.5	-4.3	4	0.625	7.000	6.000	0.188	0.0
5	D	11	2.8	-4.2	12	-2.6	-3.3	4	0.625	7.000	6.000	0.188	0.0
		1	0.0	4.4	11	2.8	-4.2	4	0.625	7.000	6.000	0.188	0.0
5	C	8	2.8	-4.2	12	-2.6	-3.3	4	0.625	7.000	6.000	0.188	0.0
		1	0.0	4.4	8	2.8	-4.2	4	0.625	7.000	6.000	0.188	0.0
5	B	8	2.5	-4.3	12	-2.3	-3.1	4	0.625	7.000	6.000	0.188	0.0
		1	0.0	3.9	8	2.5	-4.3	4	0.625	7.000	6.000	0.188	0.0
5	A	8	0.0	-1.8	8	0.0	-1.8	4	0.625	7.000	6.000	0.188	0.0
		1	0.0	1.7	8	0.0	1.7	4	0.625	7.000	6.000	0.188	0.0

NOTES FOR REACTIONS

- All loading conditions are examined and only maximum/minimum H or V and the corresponding H or V are reported.
- Positive reactions are as shown in the sketch. Foundation loads are in opposite directions.
- Bracing reactions are in the plane of the brace with the H pointing away from the braced bay. The vertical reaction is downward.
- Building reactions are based on the following building data:
  - Width (ft) = 60.0
  - Length (ft) = 100.0
  - Eave Height (ft) = 18.0/ 18.0
  - Roof Slope (rise/12 ) = 1.0/ 1.0
  - Dead Load (psf ) = 1.7
  - Collateral Load (psf ) = 3.0
  - Roof Live Load(psf ) = 20.0
  - Frame Live Load(psf ) = 12.0
  - Wind Speed (mph ) = 130.0
  - Wind Code = IBC 09
  - Exposure = B
  - Closed/Open = C
  - Importance Wind = 1.00
  - Importance Seismic = 1.00
  - Seismic Design Category = A
  - Seismic Coeff (Fa\*Sa) = 0.09
- Loading conditions are:
  - 1 DL+CL+LL
  - 2 0.60DL+WL1
  - 3 0.60DL+WR1
  - 4 0.60DL+LnWnd1+LWIND1\_L2E
  - 5 0.60DL+LnWnd1+LWIND1\_R2E
  - 6 0.60DL-LnWnd1+LWIND1\_L2E
  - 7 0.60DL-LnWnd1+LWIND1\_R2E
  - 8 0.60DL+WR2+WS
  - 9 0.60DL+WP+LnWnd2
  - 10 DL+CL+0.75SL+0.75WR2+0.75WS+0.75Slide
  - 11 0.60DL+WL2+WS
  - 12 0.60DL+WP+LnWnd1

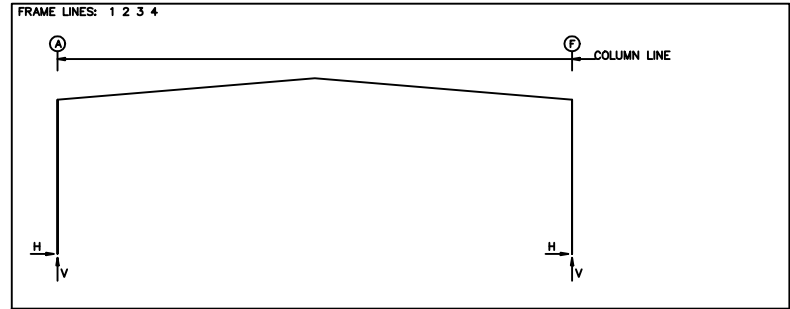
BUILDING BRACING REACTIONS

Loc	Wall Line	Col Line	± Reactions (k )				Panel Shear (lb/ft)
			Wind Horz	Wind Vert	Seismic Horz	Seismic Vert	
L_EW	1	Rigid Frame	At	Endwall			
F_SW	2,3	6.1	3.9	0.5	0.3		
R_EW	5					57	
B_SW	A	3,2	6.1	3.9	0.5	0.3	

DATE	REVISION	BY	 ROYAL Metal Building 2821 Antistad Drive San Benito, TX 78586	CUSTOMER:	JOB NO.
					Wiltour-Ellis Co. 60'x100'x18'/1:12
				DRAWN: glt	CHKD:
				DATE: 8/16/12	DATE:
					DWG. NO.

**RIGID FRAME: BASIC COLUMN REACTIONS (k)**

Frame Line	Column Line	---Dead---		---Collateral---		---Live---		---Wind_L1---		---Wind_R1---		---Wind_L2---	
		Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
1	A	1.1	2.1	1.4	2.3	5.4	9.0	-11.2	-16.4	-3.7	-11.0	-9.2	-9.4
1	F	-1.1	2.1	-1.4	2.3	-5.4	9.0	3.7	-11.0	11.2	-16.4	1.7	-4.1
Frame Line	Column Line	---Wind_R2---		---LnWind1---		---LnWind2---		---Selsmic_L---		---Selsmic_R---		---LWIND1_L2E---	
		Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
1	A	-1.7	-4.1	-6.4	-16.8	-4.3	-9.8	-0.1	-0.1	0.1	0.1	-0.3	-2.6
1	F	9.2	-9.4	6.4	-16.8	4.3	-9.8	-0.1	0.1	0.1	-0.1	0.5	-0.3
Frame Line	Column Line	---LWIND1_R2E---		---LWIND2_L2E---		---LWIND2_R2E---							
		Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
1	A	-0.5	-0.3	-0.3	-2.6	-0.5	-0.3						
1	F	0.3	-2.6	0.5	-0.3	0.3	-2.6						
Frame Line	Column Line	---Dead---		---Collateral---		---Live---		---Wind_L1---		---Wind_R1---		---Wind_L2---	
		Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
2*	A	1.1	2.1	1.4	2.3	5.4	9.0	-11.2	-16.4	-3.7	-11.0	-9.2	-9.4
2*	F	-1.1	2.1	-1.4	2.3	-5.4	9.0	3.7	-11.0	11.2	-16.4	1.7	-4.1
Frame Line	Column Line	---Wind_R2---		---LnWind1---		---LnWind2---		---Selsmic_L---		---Selsmic_R---		---LnSels---	
		Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
2*	A	-1.7	-4.1	-6.4	-20.7	-4.3	-13.7	-0.1	-0.1	0.1	0.1	0.0	-0.3
2*	F	9.2	-9.4	6.4	-20.7	4.3	-13.7	-0.1	0.1	0.1	-0.1	0.0	-0.3
Frame Line	Column Line	---LWIND1_L2E---		---LWIND1_R2E---		---LWIND2_L2E---		---LWIND2_R2E---					
		Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
2*	A	-0.3	-2.6	-0.5	-0.3	-0.3	-2.6	-0.5	-0.3				
2*	F	0.5	-0.3	0.3	-2.6	0.5	-0.3	0.3	-2.6				
2* Frame lines:		2 3 4											



**RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES**

Frm Line	Col Line	Load ID	Column Reactions (k)				Anc. Bolt Qty	Base Plate (in)			Grout (in)	
			Hmax	V	Hmin	V		Width	Length	Thick		
1	A	1	7.9	13.3	2	-10.6	4	0.750	8.000	8.125	0.500	0.0
					6	-6.0						
1	F	3	10.6	-15.1	1	-7.9	4	0.750	8.000	8.125	0.500	0.0
		1	-7.9	13.3	7	6.0						

**RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES**

Frm Line	Col Line	Load ID	Column Reactions (k)				Anc. Bolt Qty	Base Plate (in)			Grout (in)	
			Hmax	V	Hmin	V		Width	Length	Thick		
2*	A	1	7.9	13.3	2	-10.6	4	0.750	8.000	8.125	0.500	0.0
					4	-6.0						
2*	F	3	10.6	-15.1	1	-7.9	4	0.750	8.000	8.125	0.500	0.0
		1	-7.9	13.3	5	6.0						
2* Frame lines:		2 3 4										

DATE	REVISION	BY		CUSTOMER:		JOB NO.
				Wiltour-Ellis Co.	60'x100'x18'/1:12	
				DRAWN: glt	CHKD:	DWG. NO.
				DATE: 8/16/12	DATE:	